

777 SOUTH WESTERN HIGHWAY, BYFORD STORMWATER MANAGEMENT PLAN

PREPARED FOR TAL GP PROJECTS



Ordinary Council Meeting - 15 July 2024

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HISTORY AND STATUS OF THE DOCUMENT

Revision	Date issued	Author	Issued to	Revision type
Rev A	26/02/2024	Shane Highman	Planning Solutions	Initial Review
Rev B	12/06/2024	Shane Highman	Planning Solutions	Final DA Submission



1.0 INTRODUCTION

Porter Consulting Engineers has been engaged by TAL GP Projects to prepare a stormwater management plan for the proposed commercial development on Lot 777 South Western Highway in Byford.

The site is approximately 8,700m² in size and is located directly west of the Walters Road-South Western Highway intersection. The site's location is shown in Figure 1.



Figure 1 – Site Layout

2.0 DEVELOPMENT LAYOUT

The intent is to develop the site into a storage facility. The current site plan is presented in **Appendix A**.

3.0 THE EXISTING SITE

3.1 Landform

The site contains a slender building with a large shed behind, historical aerial photography suggests this was built prior to 1974.



The site has a sealed pavement with crossovers along the South Western Highway frontage and what appears to be a laydown area at the rear. The site is vegetation free.

A feature survey is presented in **Appendix B**.

A geotechnical investigation for this site is not available, however based on nearby sites, it is expected the ground consists of a Gravelly Sandy Clay at depth.

George Street is being upgraded by MetroNet. This scope includes a below ground drainage network.

4.0 EARTHWORKS

Due to the lay of the land and configuration of the development, cutting, filling and retaining is required. Concept building and road levels are documented on the Stormwater Management Plan as presented in **Appendix C**. The exact details and extent of retaining will be resolved at building license stage.

5.0 STORMWATER STRATEGY

It is anticipated the ground conditions do not permit onsite soakage therefore the development will utilize onsite detention and establish a formal connection to the proposed drainage network on George Street. The size of the detention tanks and post development off-site flow will be to the Shire of Serpentine Jarrahdale's requirements.

- Permissible discharge rate off site is pre-development 1 in 5 ARI
- Detention tanks manage the 1% AEP
- The first 15mm of rainfall will be treated via bioretention zones.
- Drainage system to be emptied in 96 hours
- No runoff into neighboring properties

6.0 PROPOSED STORMWATER SYSTEM

As typical with any built form development, this project will include down pipe connections that will join into the carpark drainage network. This carpark network will drain into a bioretention zone before flowing into the detention tanks. The tanks will have a low flow outlet that will ultimately discharge into the George Street network.

It is likely that subsoil drainage will be required under the bioretention zones and in areas of cut, and potentially along the backs of any fill retaining walls to manage the perched ground water. The specifics of this will be resolved at detailed design stage.

The above is summarized and presented in the Stormwater Management Plan (Appendix C).



7.0 STORMWATER CALCULATIONS

The Stormwater drainage calculations are tabled on the Stormwater Management Plan (Appendix C) with the key items summarised below.

- Catchment Area = $8,680m^2$
- Detention Volume = $221m^3$
- Bioretention Volume = $130m^3$
- Post Development Outflow = 39 1/s
- Outlet orifice size = 140mm

The critical duration event is based on the time taken for the whole site to contribute.

The site length and width is 100m x 80m, therefore the travel distance is 180m.

Based on F2.10 [2.3] of the Hydraulics Precast Conduits and using a post-development grade of 1% (will be much steeper) on a paved surface, the expected time of concentration is about 10 minutes. This is too short and the minimum detention volume should be based on 30 minutes.

A time of concentration of 30 minutes has been used.

8.0 CONCLUSION

The site's drainage arrangements will be designed to comply with the Shire's commercial development requirements and based on the site's constraints. On site drainage facilities will be provided with an oversized detention system to limit post development flows. A connection to the George Street drainage networks will be established.

APPENDIX A - Development Layout



Revision	Amendment	Date		
А	PLANNING	12.06.2024		

FENCE TYPE LEGEND

FT1	FENCE TYPE 01 2.1m HIGH TUBULAR FENCE COLOUR: WOODLAND GREY
FT2	FENCE TYPE 02 1.5m HIGH TUBULAR FENCE COLOUR: WOODLAND GREY

DEVELOPMENT SUMMARY GROUND FLOOR AREA SCHEDULE

SITE AREA	8686m²
SITE COVERAGE	47.0%
BIN STORE	21m²
STORAGE UNITS	4090m ²
OFFICE BOX / SHOP + ACCESS WC	40m ²
GROSS INTERNAL FLOOR AREA	4151m ²
NET LETTABLE AREA (STORAGE UNITS)	4090m²
NLA EFFICIENCY (NET LETTABLE AREA)	98.5%
NO. OF UNITS	375
AVERAGE UNIT SIZE	10.9m ²
% OF DRIVE UP UNITS -	75.5% (3088m² ARE/
% OF WALK UP UNITS	24.4% (1002m² ARE/
CARPARKING ZONES LENGTH	169.5m APPR
LANDSCAPED SPACE REQUIRED : MIN. 10% OF SITE AR AREA REQUIRED = 868	EA
AREA SUPPLIED = 868m ² = 10%	
NOTE: INCLUDES ROAD WIDENING AS LANDSCAPING	

TAL GP: BYFORD

DRIVE UP & W	ALK UP UNITS			
SIZE	No. OF UNITS	TOTAL AREA	CUBIC AREA	ACTUAL RATIO
				BY No. OF UNITS
2 x 2	57	228 m2	547.2 m3	15.20%
2.5 x 2	6	30 m2	72 m3	1.60%
2.5 x 3	17	127.5 m2	306 m3	4.53%
3 x 2	62	372 m2	892.8 m3	16.53%
3 x 3	57	513 m2	1231.2 m3	15.20%
4 x 3	30	360 m2	864 m3	8.00%
4.5 x 3	42	567 m2	1360.8 m3	11.20%
5 x 3	20	300 m2	720 m3	5.33%
5.5 x 3	22	363 m2	871.2 m3	5.87%
6 x 3	25	450 m2	1080 m3	6.67%
7 x 3	24	504 m2	1209.6 m3	6.40%
7.5 x 3	12	270 m2	648 m3	3.20%
5.5 m2	1	5.5 m2	13.2 m3	0.27%
				100 00%

375 10.9

TOTAL No. OF STORAGE UNITS AVERAGE UNIT SIZE (m2)



202400032 S.G

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APPENDIX B - Feature Survey



APPENDIX C - Stormwater Management Plan



<u>LEGEND</u>



450/140.0

<u>D</u>/S 42.96

IL 12.55m

 \rightarrow

1.4

PROPOSED PAVEMENT LEVELS FINISHED FLOOR LEVELS CONCEPTUAL GEORGE ST DRAINAGE CONCEPTUAL METRONET GEORGE ST DESIGN EXISTING PATH EXISTING SEWER PIPE EXSITING GAS EXSITING HIGH PRESSURE GAS EXISTING WATER MAIN EXISTING POWER LINE EXISTING HIGH VOLTAGE POWER LINE EXISTING OVERHEAD POWER LINE EXISTING TELECOMMUNICATIONS

> INVERT LEVEL UP STREAM PIPE DIAMETER / GRADE LENGTH INVERT LEVEL DOWN STREAM

EXISTING GROUND CONTOUR

EXISTING GROUND SPOT LEVEL

Item 10.1.2 - Attachment 8

PROPOSED DRAINAGE OUTLET SYSTEM PIPE INVERT

PROPOSED LANDSCAPE AREA

BELOW GROUND DETENTION TANK

BIO RETENTION AREA

APPROXIMATE BIO RETENTION AREA CATCHMENT BOUNDARY

PROPOSED LOT CONNECTION PIT AND OUTLET TO STREET DRAINAGE NETWORK

CONCEPTUAL LOT DETENTION DRAINAGE

CONCEPTUAL BIO RETENTION DRAINAGE

CONCEPTUAL DOWN PIPE CONNECTION

DRAINAGE FLOW DIRECTION

<u>DESIGN SUMMARY</u>			
<u>PRE DEVELOPMENT</u>			
EY	0.2		
DESIGN STORM DURATION	30	minutes	
	39.0	ጠጠ/ ከΓ 2	
	8680	m	
	0.40 39	1/s	
		03	
POST DEVELOPMENT			
AEP	1%		
DESIGN STORM DURATION	30	minutes	
	70.7	ጠጠ/ በΓ 2	
	8680	M [−]	
FLOW- POST DEVELOPMENT	162	l/s	
<u>STORAGE</u>			
DETENTION FLOW	123	l/s	
DURATION	30	minutes	
DETENTION VOLUME	221	m ³	
OUTFLOW	39	l/s	
BIO RETENTION			
SITE AREA	8680	m ²	
RAINFALL	15	mm	
VOLUME	130	m³	
STORAGE DEPTH	0.5	m	
STORAGE AREA	260	m²	
			_

ORIFICE CALCULATION

 $Q = CA \sqrt{2 \times g \times H}$

- \therefore 0.039 = 0.6 x A x $\sqrt{2 \times 9.81 \times 1.7}$
- ∴ A = 0.0155m²
- ∴ D = 140mm

0m

1:250



15m

		1:250	DRAWING No.	REV No.	ORIGINAL DRAWING SIZE
R MANAGEMENT PLAN	DATE	Jun-24	22 11 125 /AOO B		1 1
	DESIGN	SCH	23-11-135/400	D	AI
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